Surface Water Hydrology Professor Rajib Maity Department of Civil Engineering Indian Institute of Technology Kharagpur Lecture – 55 Basics of Hydrologic Design

This week we will be discussing that hydrologic design. And this one is divided into two parts; this week we will take up this hydrologic design I. In this very first lecture, we will discuss some of the basics of this hydrologic design.

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Concepts Covere	d	
 ➢ Introduction to Hydro ➢ Hydrologic Design Setting 		
 Selection of the Design 		
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Under this concept, the introduction to hydrologic design, hydrologic design scale, and the selection of design level.

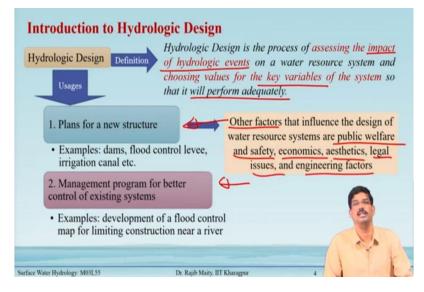
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Outline
➤Introduction to Hydrologic Design
➢Hydrologic Design Scale ▲
Estimated Limiting Value (ELV)
➤Probability Based Limits
Design for water use
Selection of the Design Level/Return Period
Empirical Approach
►Risk Analysis (Next lecture)
>Hydro-economic Analysis (Next to next lecture)
>Summary V
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The outline of our lecture goes like this, the introduction to hydrologic design, then the hydrologic design scale. Under this hydrologic design scale, we will be discussing different aspects. The first one, the most important one is the Estimated Limiting Value abbreviated as ELV. Then some probability-based limits on how to calculate those things. And what is the difference between when we talk about the higher side of the extreme and the lower side of the extreme, that comes in the case of the lower side of the extreme comes some aspect of the design up for water use?

Now, secondly that the selection of this design level or in terms of the return period, how it should, we should decide what should be the design level for a particular structure or a particular project. There are three approaches we will be discussing. In this approach after discussing the basic things, we will discuss this empirical approach. In the next lecture, we will discuss the risk analysis-based approach, and then, next lecture we will discuss the hydro-economic analysis, where the economic aspects are also considered and then we will summarize this particular lecture.

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Introduction to Hydrologic Design

Hydrologic Design is the process of assessing the impact of hydrologic events on a water resource system and choosing values for the key variables of the system so that it will perform adequately.

Usages of Hydrologic Design

1. Plans for a new structure

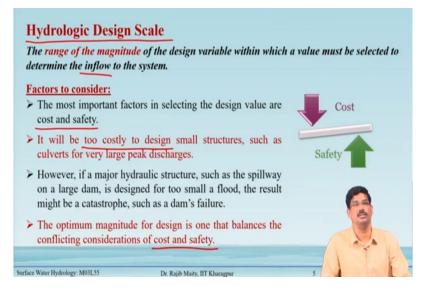
Other factors that influence the design of water resource systems are public welfare and safety, economics, aesthetics, legal issues, and engineering factors.

Examples: dams, flood control levees, irrigation canals, etc.

2. Management program for better control of existing systems

Examples: development of a flood control map for limiting construction near a river

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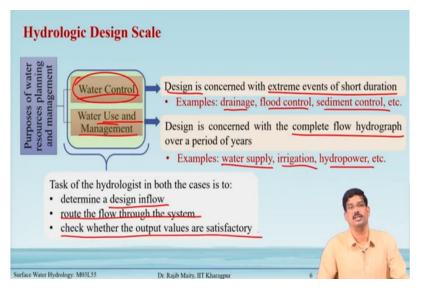
Hydrologic Design Scale

The range of the magnitude of the design variable within which a value must be selected to determine the inflow to the system.

Factors to consider:

- > The most important factors in selecting the design value are cost and safety.
- It will be too costly to design small structures, such as culverts for very large peak discharges.
- However, if a major hydraulic structure, such as the spillway on a large dam, is designed for too small a flood, the result might be a catastrophe, such as a dam's failure.
- The optimum magnitude for design balances the conflicting considerations of cost and safety.

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Purposes of water resources planning and management

i. Water control: Design is concerned with extreme events of short duration

Examples: drainage, flood control, sediment control, etc.

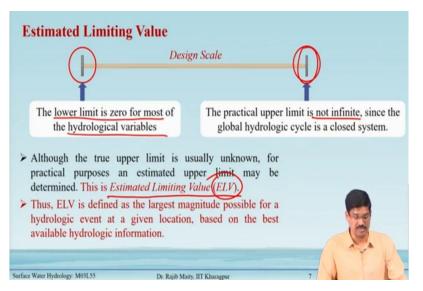
ii. Water use and management: Design is concerned with the complete flow hydrograph over a period of years.

Examples: water supply, irrigation, hydropower, etc.

The task of the hydrologist in both the cases is to:

- ➢ determine a design inflow
- ➢ route the flow through the system
- check whether the output values are satisfactory

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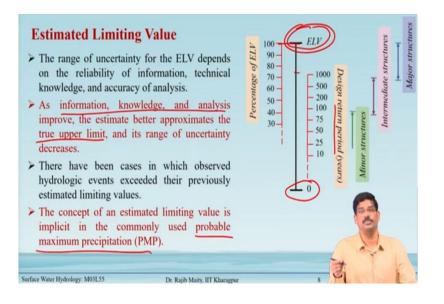


Estimated Limiting Value

In a design scale, if the lower limit is zero for most of the hydrological variables such as stream flow, the lower limit is zero then the upper limit we cannot say that it is infinite, it is not infinite, because as we know that the global hydrologic system is a close one. So, the true upper limit is generally unknown for practical purposes and the estimated upper limit may be determined this estimated upper limit is what we call the Estimated Limiting Value abbreviated as ELV.

So, definition wise ELV is defined as the largest magnitude possible for a hydrologic event at a given location, based on the best available hydrologic information.

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Design scale range from 0 to this upper limit ELV, then percentage-wise can show that this level is 100 percent of this ELV. And this gradually goes down as it goes down to the 0. Another way of describing this one is that design return period values also.

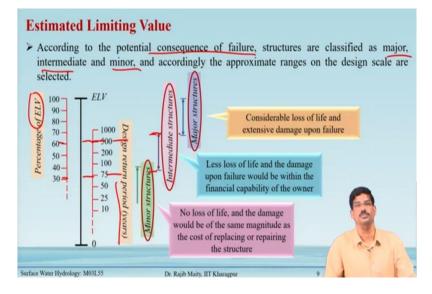
So, a particular percentage for a particular case may correspond to some of the return periods. these are some of the links from the ELV and this design return period and depending on that there are different structures are there. For example, the major structures maybe a return period of more than 500 years or so, intermediate structures have some range of these either percentage of the ELV, or through this design return period. And the minor structures may also correspond to each other.

Now, the range of the uncertainty for the ELV depends on the reliability of the information, technical knowledge, and accuracy of the analysis. As the information knowledge and analysis improve, the estimate better approximates the upper true upper limit. It is very difficult to ascertain and its range of uncertainty may decrease if this information's are very much reliable to us. There have been cases in which observed hydrologic events exceed their previously estimated limiting values.

When there are some external factors like climate change impact comes on this water resource and hydrological sciences, then this the practical upper limit that we see whatever the way we

calculated previously, that may have changed over the time. The concept of an estimated limiting value is sometimes implicit in many commonly used things. For example, the Probable Maximum Precipitation (PMP).

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According to the potential consequence of the failure for different structures, the structures can be classified as the major intermediate, or minor, accordingly, the approximate range of the design scale can be selected. For example, here when we talk about the major structures, it may correspond to the range of say 500 years of the return period or 60 percent of the ELV like that.

When we call that a major structure, there is a considerable loss of life and extensive damage if those structures fail. On the other hand, in the intermediate structures when we say the range we can see from this one maybe 500 to the 100 or 75 years return period or 30 percent of the ELV. In these cases, the less loss of life and the damage upon failure would be within the financial capacity of the owner.

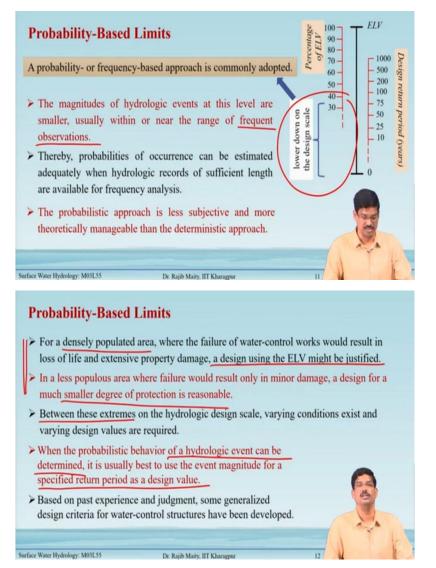
And similarly, for the minor structure, there is generally no loss of life, damage would be of the same magnitude at the cost of replacing or repairing the structure.

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Structures	Return Period (years)	Percentage of ELV	Structures	Return Period (years)	Percentage of ELV
Highway culverts	1	1	Dams with no likelihood of		
Low traffic	5-10		loss of lives (low hazard)		
Intermediate traffic	10-25	-	Small dams	50-100	-
High traffic Highway bridges	50-100	- ¥	Intermediate dams	100+	
Secondary system	10-50	-	Large dams	_	50-100%
Primary system	50-100	-	Dams with probable loss of		
Farm drainage			life (significant hazard)		
Culverts	5-50	-	Small dams	100+	50%
Ditches	5-50	-			10 1000
Urban drainage			Intermediate dams		50-100%
Storm sewers in small cities	2-25		Large dams		100%
Storm sewers in large cities	25-50	-	Dams with high likelihood of		
Airfields			considerable loss of life (high		
Low traffic	5-10	-	hazard)		
Intermediate traffic	10-25	-	Small	-	50-100%
High traffic	50-100	-			
Levees			Intermediate	-	100%
On farms	2-50	-	Large	_	100%
Around cities	50-200				

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Low traffic	5-10	—	loss of lives (low hazard)		
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High traffic	50-100		Transfer to the second se	100+	
Highway bridges			Intermediate dams	100+	
Secondary system	10-50		Large dams		50-100%
Primary system	50-100		Dams with probable loss of		
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High traffic	50-100	_			
Levees			Intermediate		100%
On farms	2-50		Large		100%
Around cities	50-200				1

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Probability-Based Limits

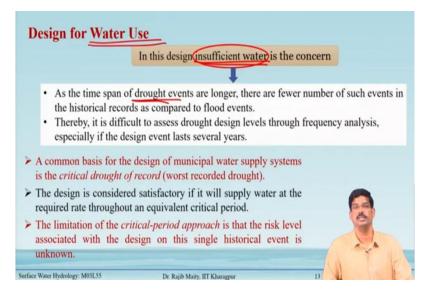
A probability- or frequency-based approach is commonly adopted because the magnitude of this hydrologic event at this level is smaller, and usually within or near the range of the frequent observations. So, in the observational record, these things can be recorded.

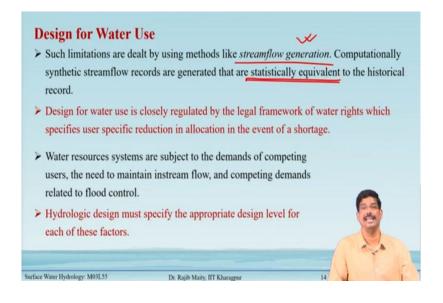
Thereby, the probabilities of the occurrence can be estimated adequately when hydrologic records of sufficient length are available for frequency analysis. The probabilistic approach is less subjective and more theoretically manageable than the deterministic approach. Now, this probability-based limit is two extremes. First of all, if the area is very highly densely populated, where the failure of the water-control works would result in the loss of life, extensive property, damage a design using the ELV might be justified. In a less populous area where the failure would result only in minor damage, a design for a much smaller degree of protection is reasonable.

Now, the first two are talking about them when it is a smaller degree of protection is reasonable. And whereas in the very highly populated one, a design that may be using the ELV might be justified, so these two are the two extremes. But, between these two extremes, the hydrologic design scale varying condition exists and varying design values are required.

Now, when the probabilistic behavior of a hydrologic event can be determined, this is one of the very important things. If those variables can capture their probabilistic behavior, it is usually best practice to use the event magnitude for a specific return period as a design value.

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Design for Water Use

In this design, insufficient water is the concern, whether it will meet the requirement or not. So, far as the insufficient water is concerned the time span of the, for example, the drought is one of the things. The drought events are longer, there is a fewer number of such events in the historical records as compared to the flood events. So, thereby it is difficult sometimes to assess the drought design levels through the frequency analysis, especially if the design events last several years or so.

A common basis for the design of the municipal water supply, for example, is a critical drought of the record, the worst recorded drought in that particular area. The design is considered satisfactory if it will supply the water at the same required rate throughout an equivalent critical period. The limitation of the critical period approach is the risk level associated with the design of this single historical event is unknown.

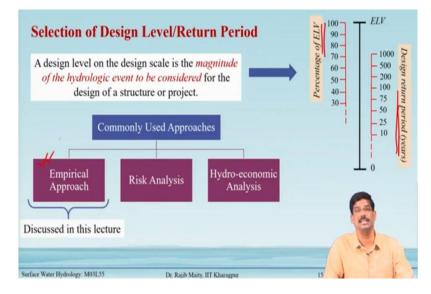
Now, such limitations can be dealt with by the user and the method like the streamflow generation. So, in this streamflow generation, again the computationally synthetic streamflow records are generated that are statistically equivalent. These are all the key terms to maintain and the use of these probability statistics methods is required in this case.

So, the statistical properties should be preserved when going for this different streamflow generation to have larger synthetic data, based on the of course based on them, or that honoring

the statistical property is in the historical record. The design of the water use is closely regulated by the legal framework of water rights, which specifies user-specific reduction in the allocation in the event of this shortage.

So, if there are multiple users are there, those things are decided on which ones should be given the more priority. Water resources systems are subject to the demand of competing interests and the need to maintain the in-stream flow, and competing demands related to flood control. Hydrologic design must specify the appropriate design level for each of this is factors.

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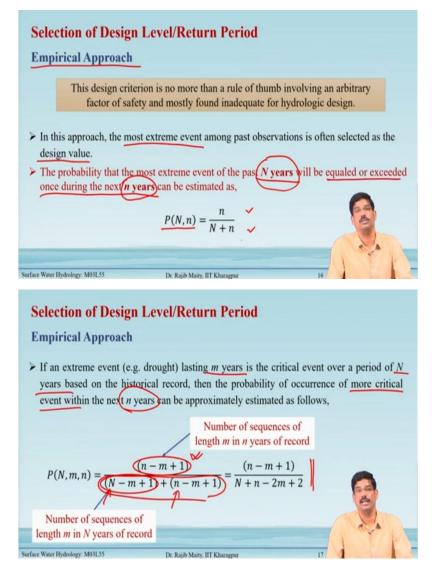


Selection of Design Level/Return Period

The selection of the design level or the return period becomes very important. So, a design level on the design scale is a magnitude that we discussed a few slides back, you can just see that it can be either in the percentage of the ELV or in terms of the return period.

The magnitude of the hydrologic event to be considered for the design of a structure or the project, so that is, so these methods are there are can be grouped into three main methods. The first one is this empirical approach. And the other two are the risk analysis and hydro-economic analysis.

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Selection of Design Level/Return Period

Empirical Approach

This design criterion is no more than a rule of thumb involving an arbitrary factor of safety and is mostly found inadequate for hydrologic design. In this approach, the most extreme event among past observations is often selected as the design value.

The probability that the most extreme event of the past N years will be equaled or exceeded once during the next n years can be estimated as,

$$P(N,n) = \frac{n}{N+n}$$

If an extreme event (e.g., drought) lasting m years is the critical event over a period of N years based on the historical record, then the probability of occurrence of a more critical event within the next n years can be approximately estimated as follows,

$$P(N,m,n) = \frac{(n-m+1)}{(N-m+1) + (n-m+1)} = \frac{(n-m+1)}{N+n-2m+2}$$

Where (n - m + 1) is the number of sequences of length m in n years of record.

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Problem 55.1	
	record at a location for the time period of <u>1971-2020</u> , it is tural drought lasts for 3.5 years. Evaluate the probability of rought in the next 25 years.
Solution	
Given, $N = 50$ years, $m = 3$ evaluated as,	3.5 years and $n = 25$ years, the required probability can be
$P(50,3.5,25) = \frac{(2)}{50+2}$	5 - 3.5 + 1) = 0.321
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Problem 55.1

Analyzing the soil moisture record at a location for the period of 1971-2020, it is observed that critical agricultural drought lasts for 3.5 years. Evaluate the probability of occurrence of a more severe drought in the next 25 years.

Solution

Given, N = 50 years, m = 3.5 years, and n = 25 years, the required probability can be evaluated as,

$$P(50,3.5,25) = \frac{(25 - 3.5 + 1)}{50 + 25 - 2 \times 3.5 + 2} = 0.321$$

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Summary

- Hydrologic design helps to choose the values of the key variables to design new water infrastructure or to assess the performance of existing water infrastructure.
- Hydrologic design scale is the range in magnitude of the design variable within which a value must be selected. This scale is used to design water control and water management structures. Design level is a value on the design scale used for hydrologic design.
- The upper limit of the design scale can be determined using deterministic as well as probabilistic approaches depending on the type of structure.
- A commonly used methods namely, empirical approach for determining the design level are discussed in this lecture.
- In the next two lectures, risk analysis and hydro-economic analysis to determine the design level/return period will be discussed one after another.

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Summary

In summary, we learned the following points from this lecture:

- Hydrologic design helps to choose the values of the key variables to design new water infrastructure or to assess the performance of existing water infrastructure.
- The hydrologic design scale is the range in magnitude of the design variable within which a value must be selected. This scale is used to design water control and water management structures. The design level is a value on the design scale used for hydrologic design.
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