

Course Name: Watershed Hydrology

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Week: 06

Lecture 30: Numerical on Synthetic UH, IUH and Distribution Graph II

**SWAYAM NPTEL COURSE ON
WATERSHED HYDROLOGY**

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Module: 06

Lecture: 05 (Numerical on Synthetic UH, IUH and Distribution Graph II)

Hello friends, welcome back to this online certification course on Watershed Hydrology. I am Rajendra Singh, a Professor in the Department of Agriculture and Food Engineering at the Indian Institute of Technology, Kharagpur. We are in Module 6, this is the last lecture, lecture number 5, and as we mentioned in the previous lecture.

Content- Numerical on Synthetic UH, IUH and Distribution Graph II

- Numerical on Synthetic Unit Hydrograph
- Numerical on Instantaneous Unit Hydrograph (IUH)
- Numerical on Distribution Graph

This will be part 2 of the Numerical on Synthetic Unit Hydrograph, IOH, and Distribution Graph. In this lecture also, we will continue with the Numerical on Synthetic Unit Hydrograph, Numerical on Instantaneous Unit Hydrograph, and Numerical on Distribution Graph.

Synthetic Unit Hydrograph

Example 1

The width of the 4-h Snyder unit hydrograph at 75% of peak is 4 h for a watershed of 250 km² area. It has a main stream that is 25 km long. The distance measured from the watershed outlet to a point on the stream nearest to the centroid of the watershed is 7 km. Calculate the time base, W_{50} , peak ordinate, and C_t for this watershed. Take C_p as 0.7.

Solution:

- Width of the 4-h Snyder unit hydrograph at 75% of peak

$$W_{75} = \frac{1.22}{q_p^{1.08}} = 4 \text{ h}$$

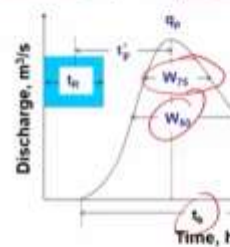
$$q_p = 0.33 \text{ cumec/km}^2$$

- Width of the 4-h Snyder unit hydrograph at 50% of peak

$$W_{50} = \frac{2.14}{q_p^{1.08}} = \frac{2.14}{0.33^{1.08}} = 7.09 \text{ h}$$

- Time base of the 4-h Snyder unit hydrograph

$$t_b = \frac{5.56}{q_p} = \frac{5.56}{0.33} = 16.85 \text{ h}$$



So, let us start with the Synthetic Unit Hydrograph. The width of a 4-hour unit hydrograph at 75 percent of peak is 4 hours for a watershed of 250 square kilometres area. It has a main stream that is 25 kilometres long. The distance measured from the watershed outlet to the point on the stream nearest to the centroid of the watershed is 7 kilometres. Calculate the time base W_{50} , Peak ordinate, and CT for this watershed, take CPH 0.7.

So, this is the problem where we have been given, you remember this is the Schneider Unit Hydrograph and these are some of the elements. One of the elements is the width at 75 percent of the peak, W_{75} , which is related to Q_p by this relationship

$$W_{75} = \frac{1.22}{q_p^{1.08}}$$

, and the value of W_{75} is given as 4 hours. So, from here, using W_{75} as 4 hours, you can get the value of Q_p as 0.33 cubic per square kilometre. And we know that the two additional ordinates, W_{50} and T_b , which were provided by US Army Corps of Engineers, are all linked to Q_p .

So, W_{50} can be calculated using this relationship,

$$W_{50} = \frac{2.14}{q_p^{1.08}}$$

and it comes out to be 7.09 hours. Similarly, the time base of the 4-hour synthetic hydrograph is again related to Q_p . So, knowing Q_p , we can calculate t_b ,

$$t_b = \frac{5.56}{q_p}$$

which comes out to be 16.85 hours. These are the three, two that is the time base and W_{50} is known to us.

Synthetic Unit Hydrograph

Solution:

- Watershed area, $A = 250 \text{ km}^2$
- The peak ordinate in terms of discharge: $Q_p = q_p A = (0.33 \times 250) \text{ cumec} = 82.5 \text{ cumec}$
- $C_p = 0.7$
- Peak discharge per unit area of the drainage basin is $q_p = \frac{2.78C_p}{t_p'} = \frac{2.78 \times 0.7}{t_p'} = 0.33 \text{ cumec/km}^2$
 $t_p' = 5.9 \text{ h}$
- Desired duration (t_R) = 4 h
- t_p can be calculated by the following equation
 $t_p = \frac{21}{22} t_p' + \frac{t_R}{4}$
 $5.9 = \frac{21}{22} t_p + \frac{4}{4}$
 $t_p = 5.13 \text{ h}$
- Length of main stream, $L = 25 \text{ km}$
- Length to Centroid $L_{ca} = 7 \text{ km}$
- C_1 can be calculated as
 $t_p = 0.75 C_1 (L L_{ca})^{0.3}$
 $5.13 = 0.75 \times C_1 \times (25 \times 7)^{0.3} \text{ h} = 5.13 \text{ h}$
 $C_1 = 1.45$

Now, the watershed area is given as 250 square kilometres. So, the peak ordinate in terms of discharge is Q_p times A , where Q_p is cubic per square kilometre already calculated as 0.33, and the area is 250 square kilometres. So, the peak discharge comes out to be 82.5 cumecs. We have been given C_p value of 0.7. So, using the relationship that is Q_p is 2.78 C_p by T_p dash because we know Q_p , we know C_p .

Using this relationship, we can calculate T_p dash, which comes out to be 5.9 hours. And we have been given the desired duration of 4 hours. So, T_p dash, we using this relationship that T_p dash is 21 by 22 T_p by T_r by 4, we can calculate the value of T_p . As we know T_p dash, we know T_r . So, knowing these two, T_p is the only unknown comes out to be 5.13 hours. We are given the length of the main stream is 25 kilometres, length to centroid is 7 kilometres. We know that this is the main relationship given by Snyder that is

$$t_p = 0.75c_t(LL_{ca})^{0.3}$$

We have been given all, we know that T_p value, we know L value, we know L Ca value. So, the only unknown is C_t here. So, by putting the values here, we can calculate C_t value, which comes out to be 1.45. With this way, we have answered all the bits of this particular problem.

SCS Triangular Unit Hydrograph Method

Example 2

A catchment with an area of 756 km² has a 6 h unit hydrograph which is triangular with a base of 70 h. Calculate the peak discharge of direct runoff hydrograph due to 5 cm of rainfall excess in 6 h from the catchment. (GATE 2008)

Solution:

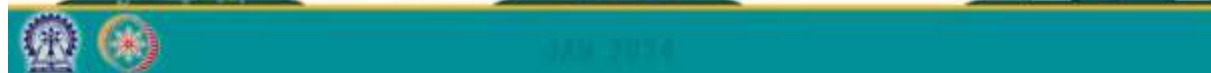
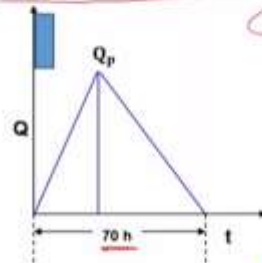
- Area of the basin (A) = 756 km²
- Duration of excess rainfall (D) = 6 h
- Time base (t_b) = 70 h
- Peak discharge of 6-h UH can be calculated as

$$\frac{1}{2} \times t_b \times Q_p = ER \times A$$

$$\frac{1}{2} \times 70 \times 3600 \times Q_p = \frac{1}{100} \times 756 \times 10^6$$

$$Q_p = 60 \text{ m}^3/\text{s}$$

- Hence, the peak discharge of the DRH due to 5 cm of rainfall excess = $60 \times 5 = 300 \text{ m}^3/\text{s}$



Then we go to the second problem, SC, which is on SCS triangular unit hydrograph method, which is example 2 of this lecture. A catchment with a catchment area of 750 square kilometres has a 6-hour unit hydrograph, which is triangular with a base of 70 hours. Calculate the peak discharge of direct runoff hydrograph due to 5 centimetres of rainfall excess 6 hours from the catchment. This question is taken from GATE 2008 examination.

So, here we have been given the time base of 75 70 hours, the area of the basin is given, and the duration of excess rainfall is given. We know this relationship that area under the curve represents the volume of runoff. So, the area is half T_b times Q_p , and we also know that a unit hydrograph means 1 unit depth of effective rainfall over the entire basin area.

So, Er times a . So, here T_b is given as 70. So, we are putting 70, but it is in hours. So, we are converting it into seconds. Q_p is the only unknown here. Er is 1 centimetre. So, that means, in meters we are putting 1 by 100, and area is given as 750 square kilometres.

So, we are putting 750 into 10 to the power 6 square meters and then solving for we get Q_p equals to 60 cubic meters per second. So, the peak discharge of the DRH due to 5 centimetres of this is the peak discharge of this unit hydrograph. So, for DRH, we need to multiply with

effective rainfall magnitude. So, peak of discharge of DRH due to 5 centimetres of rainfall will be 60 times effective rainfall that is 5 centimetres. So, the answer comes out to be 300 cubic meters per second. So, a pretty simple problem which was asked in the GATE 2008 examination.

Dimensionless Unit Hydrograph

Example 3

Derive a 5-h unit hydrograph ordinate by using the dimensionless unit hydrograph method considering time to peak 5 h and peak discharge of 3 m³/s.

Solution:

- Time to peak (t_p) = 5 h
- Peak discharge (q_p) = 3 m³/s
- Steps for deriving UH from DUH
 - Select the time interval of 5 h and calculate t/t_p
 - Determine the corresponding value of q/q_p from the dimensionless unit hydrograph table
 - Multiply the values of q/q_p by Peak discharge (q_p)
 - Finally, construct the unit hydrograph

Handwritten notes: $t/t_p \rightarrow 0-5$, $t=0$ to 25

We go to example number 3, which is on the dimensionless unit hydrograph. Derive a 5-hour unit hydrograph ordinate by using the dimensionless unit hydrograph method considering time to peak 5 hours and peak discharge of 3 cubic meters per second. So, time to peak T_p is given here, peak discharge is given here. The steps we have already seen in previous problems also select a time interval of 5 hours and calculate T by T_p . Remember T by T_p has a range of 0 to 5.

So, that means, T value will go from T will be from 0 to 25. So, every 5 hours, we have to determine the corresponding values of Q by Q_p from the dimensionless unit hydrograph table, which is provided by SCS or NRCS multiply the values of Q by Q_p by peak discharge Q_p , which is given to us to get the ordinate ordinates of the unit hydrograph and then concentrate the unit hydrograph.

Dimensionless Unit Hydrograph

Solution: $t_p = 5 \text{ h}$, $q_p = 3 \text{ m}^3/\text{s}$

Time Ratios (t/t_p)	Discharge Ratios (q/q_p)	Mass Curve Ratios (Q_p/Q)
0	.000	.000
1	.030	.001
2	.100	.006
3	.190	.012
4	.310	.035
5	.470	.065
6	.660	.107
7	.820	.163
8	.930	.228
9	.990	.300
1.0	1.000	.375
1.1	.990	.450
1.2	.930	.522
1.3	.860	.589
1.4	.780	.650

Time Ratios (t/t_p)	Discharge Ratios (q/q_p)	Mass Curve Ratios (Q_p/Q)
4.0	.011	.997
4.5	.005	.999
5.0	.000	1.000

Time (h), t	t/t_p	q/q_p	$q \text{ (m}^3/\text{s)}$
0	0	0	0
5	1	1	3
10	2	0.28	0.84
15	3	0.055	0.165
20	4	0.011	0.033
25	5	0	0

Based on the value of t/t_p , q/q_p is obtained from the standard dimensionless UH Table

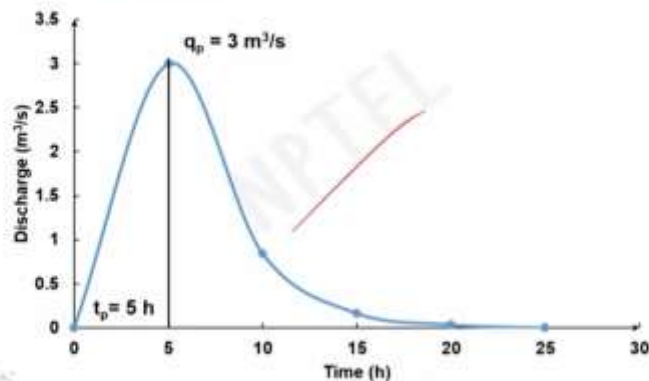
So, T_p is given as 5 hours Q_p is given 3 cubic this is the truncated SCS standard dimensionless unit hydrograph table. So, where we said that time will vary range from 0 to 35 because T by T_p will be max value is 5 which ranges between 0 and 5. So, 0 1 2 3 4 5 and thus we can because it these values are all integers straight away, we can read the values from the table like for 1 it is 1 in this table it is 4 is coming here 4 value is 0.

With intermediate values, I have not shown here, but you can straight away get all these values from the table itself. So, that means, knowing T by T_p , we are reading these values from Q by Q_p values from this standard table and putting here, and then Q_p will be nothing but these multiplied by Q_p , that is the Q value. So, obviously, we are multiplying by Q_p , which comes out to be $3Q$, which is given as 3 cubic meters per second.

So, 3.84 and so on. So, the other ordinates of this unit hydrograph.

Dimensionless Unit Hydrograph

Solution: • 5-h UH is plotted below using the table value



This is a 5-hour unit hydrograph plotted using the developed data, even the given data developed using the dimensionless standard table as well as the given data to us.

SCS Triangular Unit Hydrograph Method

Example 4

The area and time of concentration of a basin are 15 km^2 and 3h , respectively. Calculate the peak runoff rate for the basin from a 30 minute triangular unit hydrograph with an excess rainfall of 10 mm . (GATE 2004)

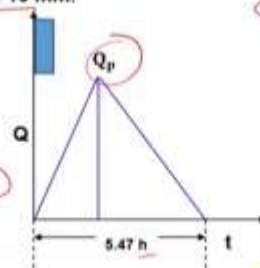
Solution:

- Area of the basin (A) = 15 km^2
- Time of concentration of a basin (t_c) = 3 h
- Duration of excess rainfall (D) = $30 \text{ min} = 0.5 \text{ h}$
- Time to peak can be calculated as, $t_p = \frac{D}{2} + 0.6t_c = \frac{0.5}{2} + 0.6 \times 3 = 2.05 \text{ h}$
- Time base can be calculated as, $t_b = 2.67t_p = 2.67 \times 2.05 = 5.47 \text{ h}$
- Peak can be calculated as

$$\frac{1}{2} \times t_b \times Q_p = ER \times A$$

$$\frac{1}{2} \times 5.47 \times 3600 \times Q_p = \frac{1}{100} \times 15 \times 10^6$$

$$Q_p = 15.23 \text{ m}^3/\text{s}$$



Then we go to the next problem, which is example 4, SCS triangle unit hydrograph method. The area and time of concentration of a basin are 15 square kilometres and 3 hours, respectively. Calculate the peak runoff rate for the basin from a 30-minute triangular unit hydrograph with an excess rainfall of 10 centimetres.

So, we have to calculate the peak runoff rate, and this question was asked in GATE 2004 examination. The area of the basin is given as 15 square kilometres, time of concentration of basin is given as 3 hours, and the duration of excess rainfall D is 30 minutes or 0.5 hours is given here. So, time to peak we know from this here we know that this is T_c T_p is related to D

by 2 and Tc by this relationship that is D would D by 2 plus 0.6 Tc D is given known to us Tc is known to us.

So, by putting these values here, we can calculate the value of Tp which comes out to be 2.05 hours, and time base is 2.67 Tp that is a standard triangular unit hydrograph relationship we know. Because Tp is calculated so we know the time base is 5.47 hours. Now, once we know that for obtaining the value of Qp we have to use the concept that area under the curve is equal to the volume of runoff generated by 1 centimetre of effective rainfall. So, half into Tv into Qp, that is the area under the curve and is equal to Er, which is 1 centimetre. So, 1 by 100 meters and area is 15 square kilometres.

So, 15 into 10 to the power 6 square meters. So, that gives us a value of Qp equals to 15.23 cubic metre per second, that is the answer to the particular question. So, it is a very straightforward question.

SCS Triangular Unit Hydrograph Method

Example 5

A triangular 5-h unit hydrograph (UH₁) of a catchment area of 400 km² has a peak discharge of 60 m³/s. Another triangular 5-hour unit hydrograph (UH₂) having the same base width as UH₁ has a peak flow of 90 m³/s. Calculate the catchment area corresponding to UH₂ in km².

(GATE 2014)

Solution:

- Catchment area of UH₁ (A₁) = 400 km²
- Peak discharge of UH₁ (Q_{p1}) = 60 m³/s
- Peak discharge of UH₂ (Q_{p2}) = 90 m³/s

The time base of the UH₁ can be calculated as

$$\frac{1}{2} \times t_b \times Q_{p1} = ER \times A_1$$

$$\frac{1}{2} \times t_b \times 3600 \times 60 = \frac{1}{100} \times 400 \times 10^6$$

$$t_{b1} = 37.04 \text{ h} = t_{b2}$$

(time base of both UHs is the same)

Catchment area of the basin having UH₂ can be calculated as

$$\frac{1}{2} \times t_{b2} \times Q_{p2} = ER \times A_2$$

$$\frac{1}{2} \times 37.04 \times 3600 \times 90 = \frac{1}{100} \times A_2 \times 10^6$$

$$A_2 = 600 \text{ km}^2$$

Now, we go to example number 5, which is again on the triangular unit hydrograph method. A triangular 5-hour unit hydrograph UH₁ of a catchment area of 400 square kilometres has a peak discharge of 60 cubic meters per second. Another triangular 5-hour unit hydrograph UH₂ having the same base as UH₁ has a peak flow of 90 cubic meters per second. Calculate the catchment area corresponding to UH₂ in square kilometres, and this question has been taken from GATE 2014 examination. So, the catchment area value information provided is a catchment area of UH₁ is 400 square kilometres, which we are calling A₁. Peak discharge of UH₁, which we are calling Q_{p1}, is 60 cubic meters per second, and peak discharge of UH₂, Q_{p2}, is 90 cubic meters per second. So, the time base of the unit hydrograph can be calculated by the same formulation, the same concept that the area under the curve represents the volume of runoff generated because of 1 centimetre of effective rainfall over the catchment area.

So, here Q_{p1} value is given to us as 60 cubic meters per second. So, here, and we want the answer in hours. So, we are calculating the hours itself, and here 1 by 100 times the area of the basin in square meters. So, putting this, we get the time base of the hydrograph as 37.04 hours, which is the same as the time base of the second unit hydrograph also, because it says that the

time base of the two hydrographs is the same, both are 5 hours, and the time base is the same, only the peak values are 60 and 90. The first area is 400 square kilometres, and we have to find the second area. So, again using the same concept, that is half times T_{b2} times Q_{p2} is E_r times A_2 , A_2 is the unknown here. So, half into T_b , time base is 37.05 04 hours, so that is in seconds. Q_{p2} is 90, so we are putting it here, and E_r is 1 by 100, and A_2 again in square meters or square kilometres. So, we are putting it here into 10 to the power 6. So, from here, we get the value of A_2 as 600 square kilometres, which is the answer to this particular question, that the area of the second catchment is 600 square kilometres.

Time Area Method

Example 6

Table 1 presents the runoff hydrograph for a catchment, and Table 2 presents the average rainfall intensity during a storm.

Using the data

- Calculate the catchment area using the time-area method
- Plot the time-area histogram for the catchment
- Compute the runoff hydrograph ordinates and plot it

Table 1. Runoff hydrograph

Time (min)	Runoff (ha-mm/h)
0	$Q_0 = 0$
5	$Q_1 = 170$
10	$Q_2 = 440$
15	$Q_3 = 1210$
20	$Q_4 = 2380$
25	$Q_5 = 2450$

Table 2. Average rainfall intensity

Time (min)	Intensity (mm/h)
0	$i_0 = 50$
5	$i_1 = 40$
10	$i_2 = 50$
15	$i_3 = 40$
20	$i_4 = 20$
25	$i_5 = 0$

We go to the next problem, which is on the time-area method. This is the first problem we are going to discuss on this particular method. Table 1 presents the runoff hydrograph for a catchment, and table 2 presents the average rainfall intensity during a storm. So, these are the two tables here. Calculate the catchment area using the time-area method. Plot the time-area histogram of the catchment. Compute the runoff hydrograph ordinates and plot it. So, earlier, in the previous problems, we were given the area under the histogram, but in this case, we are given the discharge represented by different hydrograph histograms, that is isochrones, and the average rainfall intensity's time duration is given. ΔT value should be the same, which is 5 minutes in both cases. So, that is what it is. So, basically, we now have to find out these contributing areas.

Time Area Method

Solution:

Time (min)	Contributing area (ha)
0-5	a_1
5-10	a_2
10-15	a_3
15-20	a_4
20-25	a_5

Time (min)	Intensity (mm/h)
0	$i_0 = 50$
5	$i_1 = 40$
10	$i_2 = 50$
15	$i_3 = 40$
20	$i_4 = 20$
25	$i_5 = 0$

Time (min)	Runoff (ha-mm/h)
0	$Q_0 = 0$
5	$Q_1 = 170$
10	$Q_2 = 440$
15	$Q_3 = 1210$
20	$Q_4 = 2380$
25	$Q_5 = 2450$

Runoff hydrograph ordinates (ha-mm/h) equation is given as

$$Q_i = \sum_{j=1}^i a_j i_{i-j}$$

$$Q_0 = a_0 i_0 = 0$$

$$Q_1 = a_1 i_0 = 170$$

$$Q_2 = a_1 i_1 + a_2 i_0 = 440$$

$$Q_3 = a_1 i_2 + a_2 i_1 + a_3 i_0 = 1210$$

$$Q_4 = a_1 i_3 + a_2 i_2 + a_3 i_1 + a_4 i_0 = 2380$$

$$Q_5 = a_1 i_4 + a_2 i_3 + a_3 i_2 + a_4 i_1 + a_5 i_0 = 2450$$

So, let us say that for different isochrones the areas are A1, A2, A3, A4, A5, that is for isochrones represented by 5 minutes 5 to 10, 10 to 15, 15 to 28, 20 to 25. Intensity values are already given to us and runoff value is also given, and you know that in earlier problems we were calculating runoff using this relationship, and we also remember that we said that there will be a time lag because the first intensity will occur in the first area and reach the ordinate at the first hour, at the second hour the second will contribute, third hour three will contribute, and so on and so forth three different areas will contribute. So, this is how the relationship will flow. So, at 0 there is no problem. Q1 will be the first area due to the first rainfall intensity, that is I0.

So, that value is given as 170. Q2, where A1 will have the second rainfall intensity, that is I1, and second area will have I0, and that value is given as 440. Similarly, Q3 is A1 I2 A2 I1, and now the third will also come into picture with I1, so that value is given at 1210. Q4 is A1 I3 A2 I2 A3 I1 A4, fourth area will also come, which is given as 2380. So, this is 2380, and Q5 value is 2450. So, A1 I4 A2 I3 A3 I2 A4 I1 A5 I0.

Time Area Method

Solution:

$$Q_0 = a_0 i_0 = 0$$

$$a_0 \times 0 = 0$$

$$a_0 = 0 \text{ ha}$$

$$Q_1 = a_1 i_0 = 170$$

$$a_1 \times 50 = 170$$

$$a_1 = 3.4 \text{ ha}$$

$$Q_2 = a_1 i_1 + a_2 i_0 = 440$$

$$a_1 i_1 + a_2 i_0 = 440$$

$$3.4 \times 40 + a_2 \times 50 = 440$$

$$a_2 = 6.08 \text{ ha}$$

$$Q_3 = a_1 i_2 + a_2 i_1 + a_3 i_0 = 1210$$

$$3.4 \times 50 + 6.08 \times 40 + a_3 \times 50 = 1210$$

$$a_3 = 15.94 \text{ ha}$$

$$Q_4 = a_1 i_3 + a_2 i_2 + a_3 i_1 + a_4 i_0 = 1380$$

$$3.4 \times 40 + 6.08 \times 50 + 15.94 \times 40 + a_4 \times 50 = 1380$$

$$a_4 = 6.05 \text{ ha}$$

$$Q_5 = a_1 i_4 + a_2 i_3 + a_3 i_2 + a_4 i_1 + a_5 i_0 = 2450$$

$$3.4 \times 20 + 6.08 \times 40 + 15.94 \times 50 + 6.05 \times 40 + a_5 \times 50 = 2450$$

$$a_5 = 21.64 \text{ ha}$$

Time (min)	Intensity (mm/h)
0	$i_0 = 50$
5	$i_1 = 40$
10	$i_2 = 50$
15	$i_3 = 40$
20	$i_4 = 20$
25	$i_5 = 0$

After incorporating the values of contributing areas, other runoff hydrograph ordinates (ha-mm/h) can be calculated

$$Q_6 = a_2 i_4 + a_3 i_3 + a_4 i_2 + a_5 i_1 = 1927.3$$

$$Q_7 = a_3 i_4 + a_4 i_3 + a_5 i_2 = 1642.8$$

$$Q_8 = a_4 i_4 + a_5 i_3 = 986.6$$

$$Q_9 = a_5 i_4 = 432.8$$

$$Q_{10} = a_5 i_5 = 0$$

So, putting these values, because in all the 0s this is Q_1 A_{10} is 170, only unknown is the area because I_0 value is known. So, we can calculate the value of A_1 as 3.4 hectares from this equation. A_2 will come out to be 6.08 hectares from this relationship putting the values of A_1 I_2 A_2 I_1 , and A_3 value is unknown, I_0 value is known, and right-hand side is known.

So, A_3 comes out to be 15.94, A_4 comes out to be 6.05, and A_5 comes out to be 21.64. So, different areas we have calculated, and then for the remainder of the unit hydrograph, we will continue this relationship because after 5 hours, the first rainfall, all 5 rainfalls have gone from the first area. So, that will no longer exist. For A_2 , it was A_3 I_3 . So, it is I_4 , this I_3 I_2 I_1 here, the seventh one next time step I_4 for this will go out here, and then it will be A_3 I_4 A_4 I_3 A_5 I_2 . From here, 3 will also go, only 4 and 5 will contribute.

So, A_4 I_4 A_5 I_3 , and then only the fifth will contribute, A_5 I_4 , and lastly, it will be 0. So, these values we calculate because now we know A_2 A_3 A_4 A_5 , all values are known to us. So, we calculate these values of different discharges, which were these are the calculations we have made after incorporating the values of contributing areas. Runoff hydrograph ordinates we are calculating here actually.

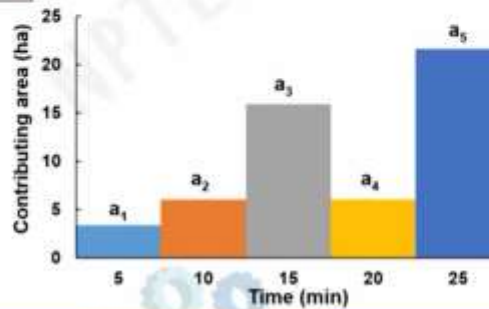
Time Area Method

Solution:

Time (min)	Contributing area (ha)
0-5	$a_1 = 3.4$
5-10	$a_2 = 6.08$
10-15	$a_3 = 15.94$
15-20	$a_4 = 6.05$
20-25	$a_5 = 21.64$

So, the catchment area is $= \sum_{i=1}^5 a_i = 53.11$ ha

Time-area histogram is plotted below

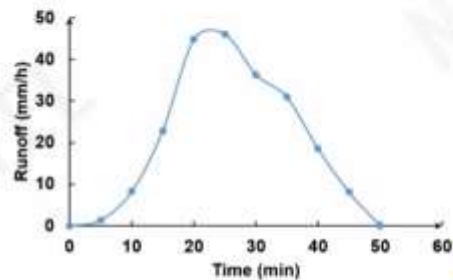


So, now, we know the contributing area, and we also know all the other answers. So, this is the first answer we have to give, that is what is the area, that is the area is 3.4, 6.08, 15.94, 6.05 and 21.64 that is plotted here for different isochrones, that is the area under isochrones is calculated here.

Time Area Method

Solution: □ Catchment area = 53.11 ha

Time (min)	Runoff (ha-mm/h)	Runoff (mm/h)
0	0	0
5	70	1.32
10	440	8.28
15	1210	22.78
20	2380	44.81
25	2450	46.13
30	1927.3	36.29
35	1642.8	30.93
40	986.6	18.58
45	432.8	8.15
50	0	0



Col.2 divided by 53.11 ha

And then we can also get the runoff in millimetres per hour because it is in hectare millimetres per hour. We know the total catchment area because we know this area. So, the sum of the areas comes out to be 53.11 hectares. So, putting these values, multiplying or dividing these runoff hectare millimetres per hour, we can get the values in one runoff in millimetres per hour. These are the ordinates and these are plotted here. We can plot in other units also, like you can calculate in cubic meters per second and also plot the hydrograph. So, the hydrograph can be plotted with different units. This is where you meet millimetres per hour. It could be in cubic

meters per second also. So, you can choose different units, but once you have calculated these runoff values here, you can convert the units the way you want and then you can plot it here. So, that is how the runoff can be obtained.

Instantaneous Unit Hydrograph (IUH)

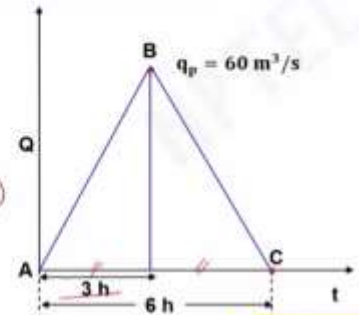
Example 7

The shape of the instantaneous unit hydrograph (IUH) of a catchment is an isosceles triangle with a peak of $60 \text{ m}^3/\text{s}$ and time to peak of 3 h. If the constant baseflow is $7.5 \text{ m}^3/\text{s}$, then find the peak of the 3-h unit hydrograph (UH) in m^3/s .

(GATE 2021)

Solution:

- ✓ Time to peak (t_p) = 3 h
- ✓ As shape of the instantaneous unit hydrograph (IUH) of a catchment is an isosceles triangle, so the time base of the hydrograph (t_b) = $2t_p = 6 \text{ h}$
- ✓ Peak of the IUH (q_p) = $60 \text{ m}^3/\text{s}$
- ✓ The slope of line AB = $\frac{60-0}{3-0} = 20 \frac{\text{m}^3}{\text{s}/\text{h}}$
- ✓ The slope of line BC = $\frac{60-0}{6-3} = 20 \frac{\text{m}^3}{\text{s}/\text{h}}$



The next problem is on instantaneous unit hydrograph. The shape of an instantaneous unit hydrograph of a catchment is an isosceles triangle with a peak of 60 cubic meters and time to peak 3 hours. If the constant base flow is 7.5 cubic meters per second, then find the peak of 3-hour hydrograph in cubic meters per second. This question is taken from GATE 2021 examination. So, we have been given time to peak is 3 hours and it is an isosceles triangle. So, the time base, of course, this will be equal to this. So, that is why time base will be $2T_p$ or 6 hours. Peak is given 60 cubic meters per second. So, we can find the slope of these lines. The slope of line AB is 60 by 0 divided by 3 , which is 20 .

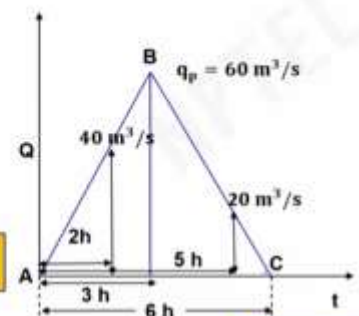
Instantaneous Unit Hydrograph (IUH)

Solution: • The ordinates of the 1-h IUH are tabulated below based on the slope AB = $20 \frac{\text{m}^3}{\text{s}/\text{h}}$ and BC = $20 \frac{\text{m}^3}{\text{s}/\text{h}}$

Time (h)	Ordinates of 1-h IUH
0	0
1	20
2	40
3	60
4	40
5	20
6	0

At t h the discharge = $q + \text{slope of AB} \cdot t$
At 2 h the discharge = $0 + 20 \cdot 2 = 40 \text{ m}^3/\text{s}$

At t h the discharge = $q_p - \text{slope of BC} \cdot t$
At 5 h the discharge = $60 - 20 \cdot 2 = 20 \text{ m}^3/\text{s}$



Similarly, the slope of line BC will also be 20 because this is 60 here and this is 0 here. So, 60 minus 0 by 3 is 20. So, same slope. So, that means, we can calculate the ordinates. It will be 0 20 40 and so on and so forth. So, 0 20 40 60 40 20 0. These will be ordinates of an instantaneous unit hydrograph. So, basically, this is just how we are calculating, but because this is a triangle. So, it is very easy to find the values here. Similarly, we can also show how to calculate the slope, but we do not need to do any fancy calculations because the slope value we know. So, we can straight away get the values here.

Instantaneous Unit Hydrograph (IUH)

Solution:

Time (h)	Ordinates of 1-h IUH	IUH (m ³ /s) lagged by 1 h	Sum of Col. 2 and Col. 3	1-h UH	1-h UH lagged by 1 h	1-h UH lagged by 2 h	3-h DRH of 3 cm ER	3-h UH
0	0	-	0	0	-	-	0	0
1	20	0	20	10	0	-	10	3.33
2	40	20	60	30	10	0	40	13.33
3	60	40	100	50	30	10	90	30
4	40	60	100	50	50	30	130	43.33
5	20	40	60	30	50	50	130	43.33
6	0	20	20	10	30	50	90	30
7	-	0	0	0	10	30	40	13.33
8	-	-	-	-	0	10	10	3.33
9	-	-	-	-	-	0	0	0

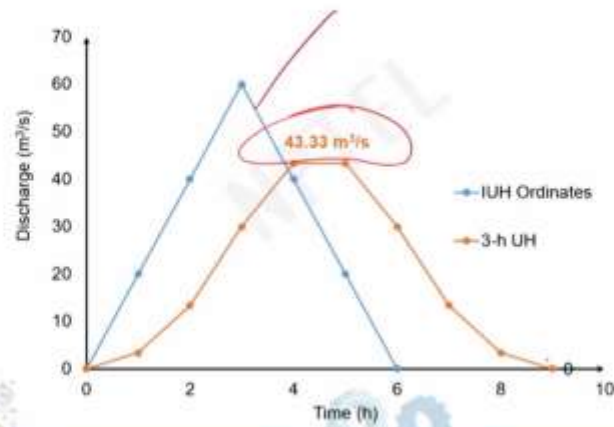
Sum of Col.2 and Col.3 is divided by 2

Peak of the 3-h UH = 43.33 m³/s

Once we have a 1-hour unit IUH, then obviously, we will first develop the 1-hour unit hydrograph, that is by lagging IUH by another hour and then taking the average. So, it is summing up the ordinates and dividing by 2, that is taking the average, that gives us a 1-hour unit hydrograph. Then we have to lag, basically we will use the matter of superposition. So, lagging by 1 hour, lagging by 2 hours, and we can then sum up the ordinates. So, here 0, 10, 40, 80, and so on. These are the 3-hour ordinates and 3-hour UH ordinates will be this divided by 3. So, 10 divided by 3 is 3.33, 40 divided by 3 is 13.33, 130 divided by 3 is 43.33, and so on. So, this is the peak of the 3-hour ordinate, which is 43.33, and there are 2 values which are coming up at the same.

Instantaneous Unit Hydrograph (IUH)

Solution: • Plots of 1-h IUH and 3-h UH are shown below.



So, this is typically the IUH ordinate, and this is the derived 3-hour unit hydrograph from this IUH. These are the ordinates here. That is how we are getting the values here.

Distribution Graph

Example 8

A 4-h unit hydrograph ordinates are given below. Find the distribution graph ordinates and plot it.

Time (h)	4-h UH
0	0
4	21
8	35
12	42
16	37
20	31
24	19
28	11
32	5
36	0

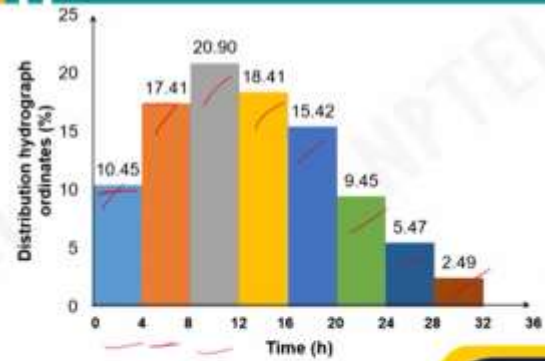
We move to the next example, which is example number 8, a 4-hour distribution graph. A 4-hour unit hydrograph ordinates are given below. Find the distribution graph ordinates and plot it. So, 4-hour unit hydrograph ordinates are given over 0 to 36 hours.

Distribution Graph

Solution:

- The distribution graph ordinates are calculated below in the table

Time (h)	4-h UH	Distribution hydrograph ordinates (%)
0	0	0
4	21	10.45
8	35	17.41
12	42	20.90
16	37	18.41
20	31	15.42
24	19	9.45
28	11	5.47
32	5	2.49
36	0	0
Summation	201	(Ordinates/Sum of the all ordinates) * 100



So, we have to obtain the distribution graph ordinates. The first thing we have to obtain is the sum of the ordinates of the given hydrograph, which we can simply sum up, which comes out to be 201. Then, the distribution hydrograph ordinates are nothing but this divided by the sum and of course, percent, so on, multiplied by 100. 21 by 201 multiplied by 100 comes out to be 10.45. So, it comes to be 17.4 and so on and so forth. So, for different distributions, we will get the distribution graph, and this is how the distribution graph is plotted here. So, it is a very simple problem. Typically, distribution graph problems are pretty straightforward. Here, different distribution graphs represent the areas differently. As we have already seen, it is just a unit hydrograph represented differently, which presents the area of the curve represented in different time intervals. 0 to 4 represents 10.45 percent, 4 to 8 represents 27.41 percent, and so on and so forth. That is how we get the distribution graph in this case.

Distribution Graph

Example 9

Distribution graph for a catchment area of 35 km² for 2 h is given in the following table. If the catchment area has a ϕ index of 2 mm/h and it receives two consecutive 2 h rainfall of 30 and 25 mm, find the ordinates of the direct runoff hydrograph and plot it.

Time interval (h)	0-2	2-4	4-6	6-8	8-10
Distribution (%)	10	15	30	25	20

We now come to the last problem, which is a distribution graph problem again, which is example number 9. The distribution graph for a catchment area of 35 square kilometres for 2 hours is given in the following table.

So, we have been given the distribution hydrograph here. In the earlier problem, we developed the distribution graph. If the catchment area has a phi index of 2 millimetres per hour and receives 2 consecutive 2-hour rainfall events of 30 and 25 millimetres, find the ordinates of the direct runoff hydrograph and plot it. This simply means that this is the unit hydrograph ordinate. Distribution hydrographs mean the unit hydrograph ordinate is given here, and we have a complex system. So, we have to operate that complex system on the unit hydrograph to get the direct runoff ordinates, and we have to plot that. That is the problem here.

Distribution Graph

Solution:

- Effective rainfall can be calculated as follows:

Time interval (h)	0-2	2-4
Rainfall in the interval (mm)	30	25
ϕ -index (mm/h)	2	2
Losses (mm)	4	4
Effective rainfall (mm)	26	21

$$ER = P - \phi \times t_e$$

- Catchment area = 35 km²

- The runoff rate corresponding to 1 cm rainfall excess is = Catchment area \times ER \times $\frac{1}{\text{Duration of ER}}$
- $$= 35 \times 10^6 \times \frac{1}{100} \times \frac{1}{2 \times 3600}$$
- $$= 48.61 \text{ m}^3/\text{s}$$

Coming to the solution, the first thing we have to calculate is the effective rainfall, that is ERS. The time intervals are given as 0 to 2 and 2 to 4, and rainfall values in the 2 events are given as 30 and 25 mm. So, these 2 events, 30 and 25 mm, are given. Also, we have been given the phi index value of 2 millimetres per hour. So, 2 times 2 millimetres per hour is given here, the phi index value. But because this is a 2-hour storm, basically the total loss will be over 2 hours, so 2 mm per hour times 2, that means, losses will be 4 and 4 millimetres in 2-hour intervals.

And so, effective rainfall is 30 minus 4, 26; 25 minus 4 is 21. So, these are the effective rainfall values in millimetres. But as you will remember, we use basically centimetres. So, this will be basically 2.6 centimetres, which we will use later on, and this will be 2.1 centimetres, which we will use later on for deriving the DRH ordinates.

The catchment area is given as 35 square kilometres. So, the runoff rate corresponding to 1 centimetre of rainfall excess is catchment area multiplied by ER multiplied by the duration of ER, which is what we already used every time. So, 35 multiplied by 10 to the power of 6, which is the area in square meters; ER is 1 centimetre per hour, so 1 by 100 meters; and the duration is 2 hours, so 2 multiplied by 3600. For 1 centimetre of rainfall, the runoff rate is 48.61 cubic meters per second. So, that is what we have obtained.

Distribution Graph

Solution: The ordinates of the direct runoff hydrograph are given below in the table

Time interval (h)	Distribution (%)	Direct runoff due to		DRH (cm)	DRH (m ³ /s)
		ER = 2.6 cm	ER = 2.1 cm		
0	0	0	-	0	0
0-2	10	0.26	0	0.26	12.64
2-4	15	0.39	0.21	0.6	29.17
4-6	30	0.78	0.32	1.10	53.23
6-8	25	0.65	0.63	1.28	62.22
8-10	20	0.52	0.53	1.05	50.80
10-12	0	0	0.42	0.42	20.42
12-14	0	0	0	0	0

$$DR = ER \times \text{Distribution}$$

$$DRH (m^3/s) = DRH (cm) \times 48.61 \frac{m^3}{s \text{ per cm}}$$

The ordinates of the direct runoff hydrograph can be obtained because here we have different hours and the distribution percentage is given. So, basically from here, we can calculate, I mean, either you can do it one way, that you first calculate the UH unit and then get the DRH ordinate, or you can simply operate on this. The effective rainfalls are 2.6 and 2.1. We know that they are from a complex storm, so there will be a lag of D hours, in this case, 2 hours, that is the thing.

So, 0 to 10, that means 2.6 centimetres, it is 0.26; 15 multiplied by 2.6, 30 multiplied by 2.6, divided by 100, actually, because of the percent. So, 26 divided by 100 is 0.26, 15 multiplied by 2.6, divided by 100 is 0.39, and so on: 0.78, 0.65, 0.72, 0.55, and so on. So, basically, we are directly using the distribution percentages and multiplying here. We could have always developed the UH ordinate first and then multiplied here, but we are doing it directly.

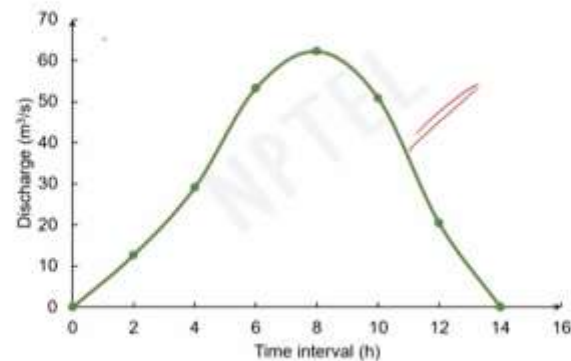
In this case, there will be a lag of D hours, that is 2 hours in this case. So, that is why the ordinate starts here.

So, 10 multiplied by 2.1, that is 21, divided by 100 is 0.21; 15 multiplied by 2.1, divided by 100 is 0.32. So, these are the DRH due to 2.1 centimetres of effective rainfall in the second 2-hour period. So, this is the first 2-hour period, and this is the second 2-hour period, and the sum of these two. This is the math principle of superposition we have studied several times and solved many problems. So, we know it is a method of superposition we are using here.

So, DRH ordinate will be the sum of these two columns. So, 0.26 here, 0.39 plus 0.21 is 0.6, 0.78 plus 0.32 is 1.10, and so on. So, already it is given here DR is ER into distribution and distribution that is percent and here it is already given DRH ordinate is in centimetres, it is given, and we know that for each centimetre, the value we have calculated is 48.61. So, we can multiply this DRH to get the DRH ordinate. We multiplied DRH in centimetres by this discharge value, which we calculated as 48.61 cubic meters per second per centimetre. So, that is why 0.26 multiplied by 48.61 is 12.64, 29.17, 59.23, and so on.

Distribution Graph

Solution: The direct runoff hydrograph (DRH) is plotted below



These are the DRH ordinates, and then we can plot these DR or DRH ordinates to get this final hydrograph. So, this is the plotted DRH, and that is how we have used a distribution graph to operate a complex system to develop this DRH.

So, I mean we have tried to handle various kinds of problems, various variations of different kinds of problems on hydrograph. And remember, in the previous module also, one lecture we entirely dedicated to numerical, and in this case too. So, enough of problems you have handled, please practice more and more problems to be efficient, to be clear on the concepts, and because while solving the problem, time-saving is very important. So, you have to be efficient. So, with this, if you have any doubt, please raise questions, and we will be happy to answer on the forum. Thank you very much.

THANK YOU